



Institute of Aeronautics and Applied Mechanics

Finite element method (FEM)

Part 3. Two dimensional (2D) cases. CST

03.2021

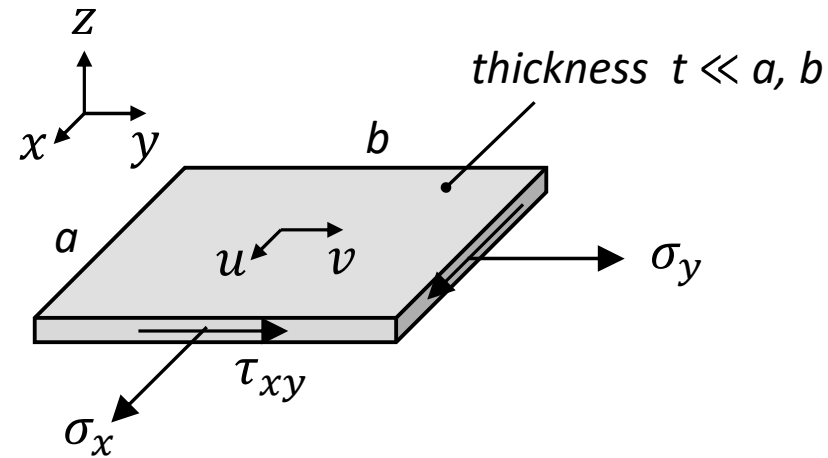
Plane stress (thin plates, shells)

$$\sigma_x ; \sigma_y ; \sigma_z = 0$$

$$\tau_{xy} ; \tau_{yz} = 0 ; \tau_{zx} = 0$$

$$\epsilon_x ; \epsilon_y ; \epsilon_z = -\frac{\nu}{E}(\sigma_x + \sigma_y)$$

$$\gamma_{xy} ; \gamma_{yz} = 0 ; \gamma_{zx} = 0$$



$$[u] = [u, v]$$

1×2

$$[\sigma] = [\sigma_x, \sigma_y, \tau_{xy}]$$

1×3

$$[\epsilon] = [\epsilon_x, \epsilon_y, \gamma_{xy}]$$

1×3

$$[D] = \frac{E}{(1-\nu^2)} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1}{2}(1-\nu) \end{bmatrix}$$

3×3
P.STRESS

$$[R] = \begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial y} \\ \frac{\partial}{\partial y} & \frac{\partial}{\partial x} \end{bmatrix}$$

3×2

$$\{\sigma\} = [D] \{\epsilon\}$$

$3 \times 1 \quad 3 \times 3 \quad 3 \times 1$

$$\{\epsilon\} = [R] \{u\}$$

$3 \times 1 \quad 3 \times 2 \quad 2 \times 1$

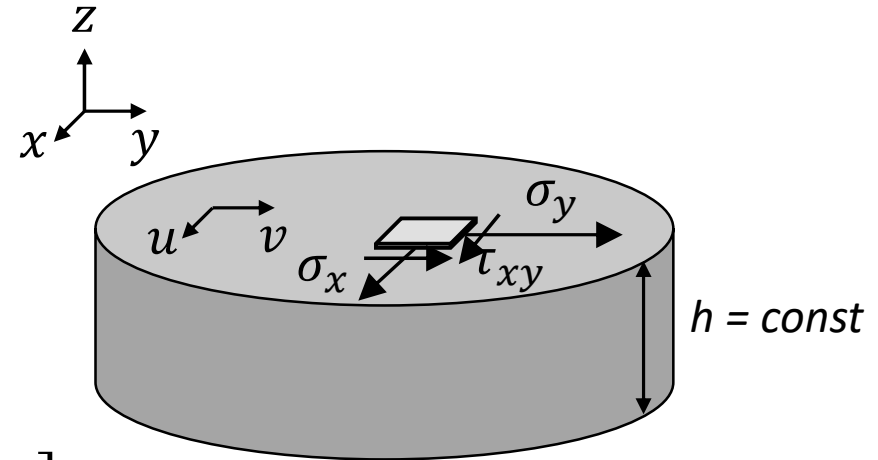
Plane strain (infinitely long pipe, prism and roller)

$$\sigma_x ; \sigma_y ; \sigma_z = \nu(\sigma_x + \sigma_y)$$

$$\tau_{xy} ; \tau_{yz} = 0 ; \tau_{zx} = 0$$

$$\epsilon_x ; \epsilon_y ; \epsilon_z = 0$$

$$\gamma_{xy} ; \gamma_{yz} = 0 ; \gamma_{zx} = 0$$



$$[u] = [u, v]$$

1 x 2

$$[\sigma] = [\sigma_x, \sigma_y, \tau_{xy}]$$

1 x 3

$$[\epsilon] = [\epsilon_x, \epsilon_y, \gamma_{xy}]$$

1 x 3

$$[R] = \begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial y} \\ \frac{\partial}{\partial y} & \frac{\partial}{\partial x} \end{bmatrix}$$

3 x 2

$$\{\sigma\} = [D] \{\epsilon\}$$

3 x 1 3 x 3 3 x 1

$$\{\epsilon\} = [R] \{u\}$$

3 x 1 3 x 2 2 x 1

$$[D] = \frac{E}{(1 + \nu)(1 - 2\nu)} \begin{bmatrix} 1 - \nu & \nu & 0 \\ \nu & 1 - \nu & 0 \\ 0 & 0 & \frac{1}{2}(1 - 2\nu) \end{bmatrix}$$

3 x 3

P. STRAIN

Axisymmetry (rotating disc)

$$\sigma_x \ ; \ \sigma_y \ ; \ \sigma_z$$

$$\tau_{xy} \ ; \ \tau_{yz} = 0 \ ; \ \tau_{zx} = 0$$

$$\epsilon_x \ ; \ \epsilon_y \ ; \ \epsilon_z = 0$$

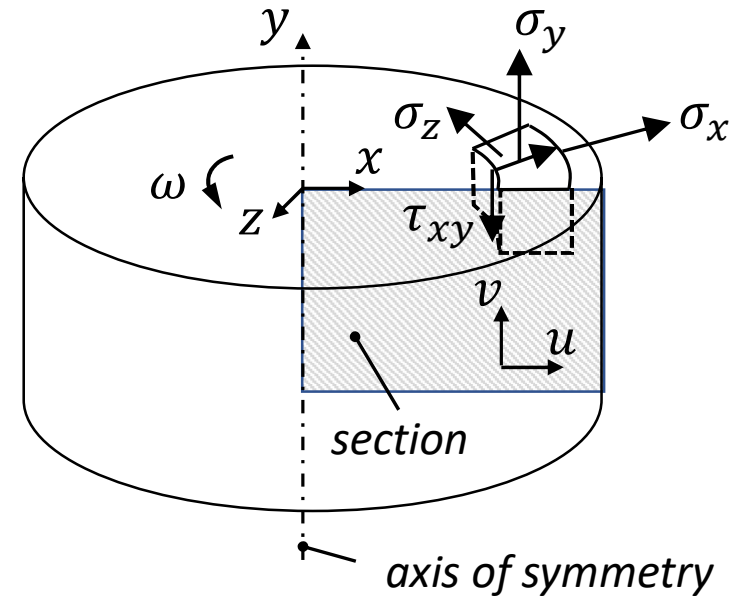
$$\gamma_{xy} \ ; \ \gamma_{yz} = 0 \ ; \ \gamma_{zx} = 0$$

$$\begin{matrix} [u] \\ 1 \times 2 \end{matrix} = [u, v]$$

$$\begin{matrix} [\sigma] \\ 1 \times 4 \end{matrix} = [\sigma_x, \sigma_y, \sigma_z, \tau_{xy}]$$

$$\begin{matrix} [\epsilon] \\ 1 \times 4 \end{matrix} = [\epsilon_x, \epsilon_y, \epsilon_z, \gamma_{xy}]$$

directions:
 x – radial
 y – longitudinal
 z – hoop



$$[R] = \begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 1 & 0 \\ x & 0 \\ 0 & \frac{\partial}{\partial y} \\ \frac{\partial}{\partial y} & \frac{\partial}{\partial x} \end{bmatrix}$$

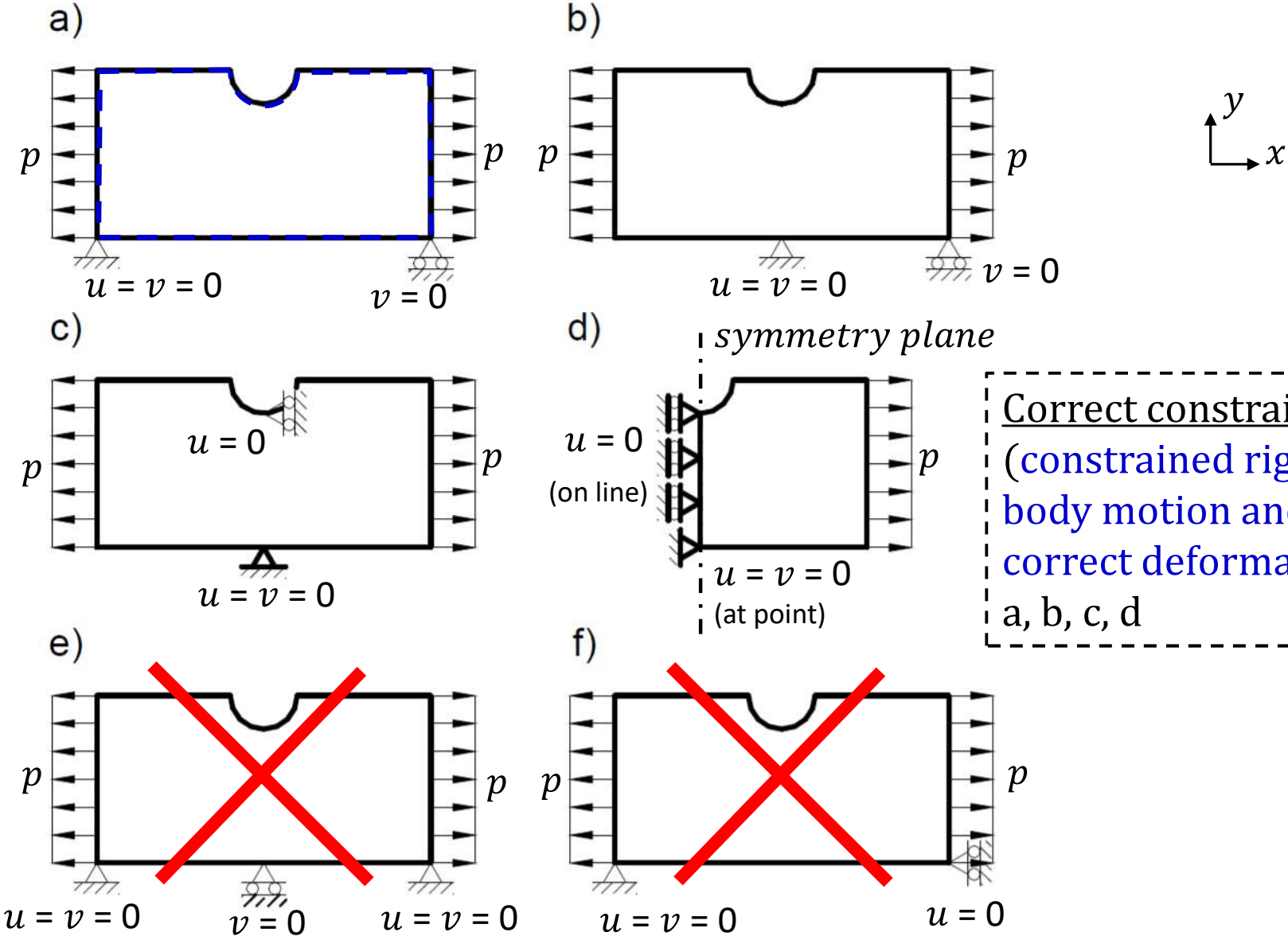
$$\begin{matrix} \{\sigma\} \\ 4 \times 1 \end{matrix} = \begin{matrix} [D] \\ 4 \times 4 \end{matrix} \begin{matrix} \{\epsilon\} \\ 4 \times 1 \end{matrix}$$

$$\begin{matrix} \{\epsilon\} \\ 4 \times 1 \end{matrix} = \begin{matrix} [R] \\ 4 \times 2 \end{matrix} \begin{matrix} \{u\} \\ 2 \times 1 \end{matrix}$$

$$[D] = \frac{E}{(1 + \nu)(1 - 2\nu)} \begin{bmatrix} 1 - \nu & \nu & \nu & 0 \\ \nu & 1 - \nu & \nu & 0 \\ \nu & \nu & 1 - \nu & 0 \\ 0 & 0 & 0 & 0.5 - \nu \end{bmatrix}$$

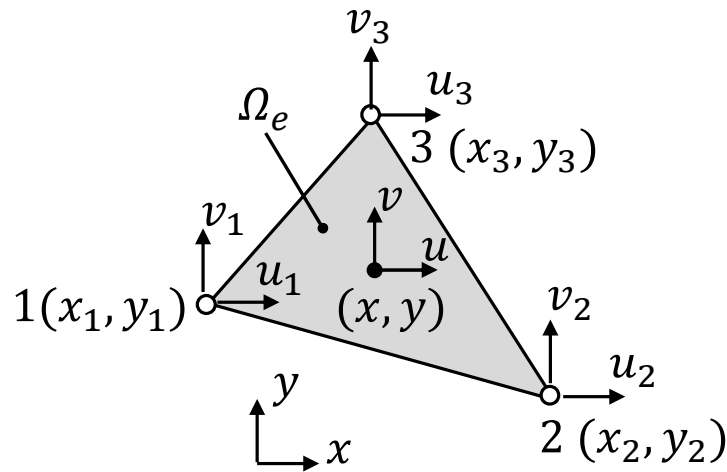
AXISYMMETRY

Constraints for a 2D plate loaded by forces being in equilibrium



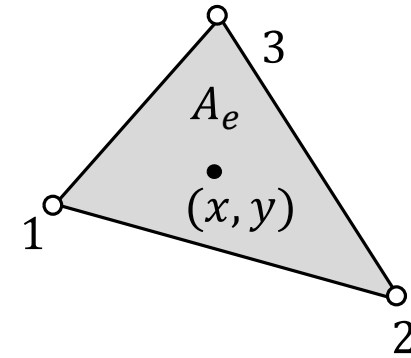
Correct constraints:
 (constrained rigid body motion and correct deformation):
 a, b, c, d

CST finite element (2D, 3-node triangle)



$$\int_{\Omega_e} d\Omega_e = A_e \cdot t_e$$

\uparrow \uparrow
 area thickness



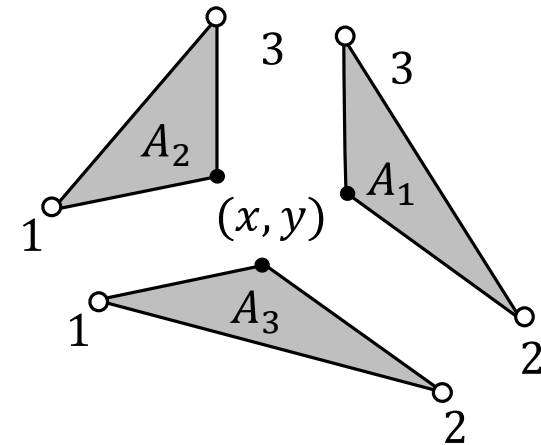
$$A_e = \frac{1}{2} \begin{vmatrix} 1 & 1 & 1 \\ x_1 & x_2 & x_3 \\ y_1 & y_2 & y_3 \end{vmatrix} = \frac{x_2 y_3 - x_3 y_2 + x_3 y_1 - x_1 y_3 + x_1 y_2 - x_2 y_1}{2}$$

$$n = 3 \quad ; \quad n_p = 2 \quad \rightarrow \quad n_e = n \cdot n_p = 6$$

Area coordinates as functions of coordinates (x, y):

$$A_e = A_1(x, y) + A_2(x, y) + A_3(x, y)$$

$$A_1 = \frac{1}{2} \begin{vmatrix} 1 & 1 & 1 \\ \bar{x} & x_2 & x_3 \\ y & y_2 & y_3 \end{vmatrix} \quad ; \quad A_2(x, y) = \frac{1}{2} \begin{vmatrix} 1 & 1 & 1 \\ x_1 & \bar{x} & x_3 \\ y_1 & y & y_3 \end{vmatrix} \quad ; \quad A_3(x, y) = \frac{1}{2} \begin{vmatrix} 1 & 1 & 1 \\ x_1 & x_2 & \bar{x} \\ y_1 & y_2 & y \end{vmatrix}$$



Shape functions of the CST element

shape functions = normalized area coordinates:

$$N_1(x, y) = \frac{A_1(x,y)}{A_e} = \frac{1}{2A_e} (a_1 + b_1x + c_1y)$$

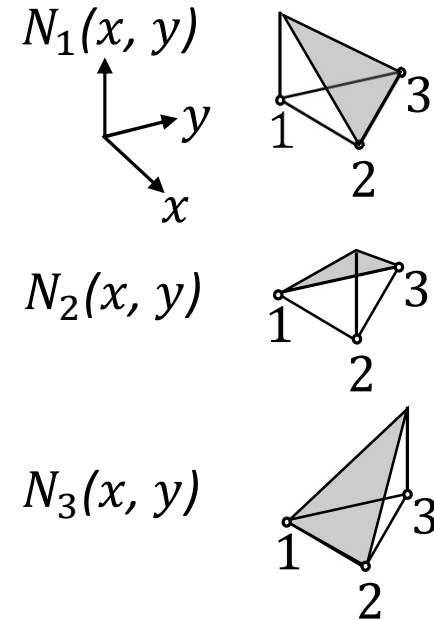
$$N_2(x, y) = \frac{A_2(x,y)}{A_e} = \frac{1}{2A_e} (a_2 + b_2x + c_2y)$$

$$N_3(x, y) = \frac{A_3(x,y)}{A_e} = \frac{1}{2A_e} (a_3 + b_3x + c_3y)$$

$$N_1(x, y) + N_2(x, y) + N_3(x, y) = 1$$

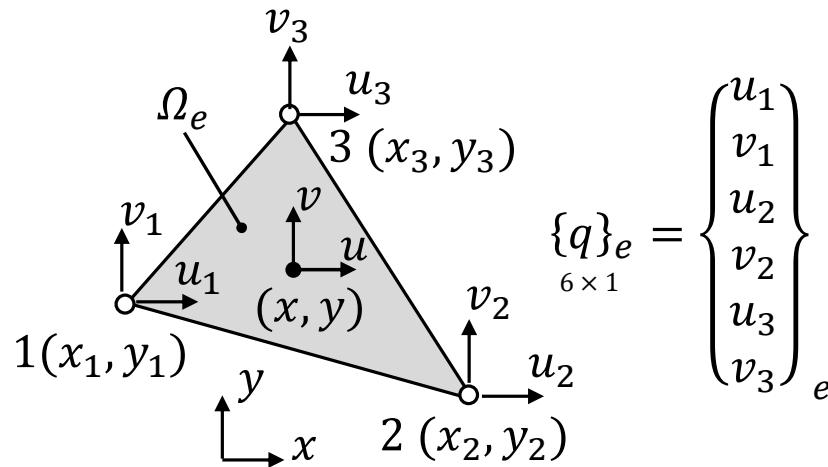
where:

$$\begin{array}{lll}
 a_1 = x_2y_3 - x_3y_2 & ; & a_2 = x_3y_1 - x_1y_3 & ; & a_3 = x_1y_2 - x_2y_1 \\
 b_1 = y_2 - y_3 & ; & b_2 = y_3 - y_1 & ; & b_3 = y_1 - y_2 \\
 c_1 = x_3 - x_2 & ; & c_2 = x_1 - x_3 & ; & c_3 = x_2 - x_1
 \end{array}$$



node	$N_1(x, y)$	$N_2(x, y)$	$N_3(x, y)$
1	1	0	0
2	0	1	0
3	0	0	1

Isoparametric mapping in the CST element



vector of shape functions:

$$[N(x, y)] = [N_1(x, y), N_2(x, y), N_3(x, y)]$$

1×3

vectors of nodal coordinates;

$$\{x_i\}_e = \begin{Bmatrix} x_1 \\ x_2 \\ x_3 \end{Bmatrix} \quad ; \quad \{y_i\}_e = \begin{Bmatrix} y_1 \\ y_2 \\ y_3 \end{Bmatrix}$$

3×1 3×1

coordinates of any point are based on shape functions and nodal parameters:

$$x = [N(x, y)] \{x_i\}_e = N_1(x, y)x_1 + N_2(x, y)x_2 + N_3(x, y)x_3$$

1×3 3×1

$$y = [N(x, y)] \{y_i\}_e = N_1(x, y)y_1 + N_2(x, y)y_2 + N_3(x, y)y_3$$

1×3 3×1

displacements at any point:

$$\{u(x, y)\} = [N(x, y)] \{q\}_e$$

2×1 2×6 6×1

Isoparametric mapping- the same shape functions used for geometry and displacements

Strain-displacement matrix of the CST element

strain vector for plane stress or plane strain conditions:

$$\begin{aligned}
 \{\varepsilon\} &= [R] \{u\} = [R] [N] \{q\}_e = \\
 &= \begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial y} \\ \frac{\partial}{\partial y} & \frac{\partial}{\partial x} \end{bmatrix} \begin{bmatrix} N_1(x,y) & 0 & N_2(x,y) & 0 & N_3(x,y) & 0 \\ 0 & N_1(x,y) & 0 & N_2(x,y) & 0 & N_3(x,y) \end{bmatrix} \{q\}_e = \\
 &= \begin{bmatrix} \frac{\partial N_1}{\partial x} & 0 & \frac{\partial N_2}{\partial x} & 0 & \frac{\partial N_3}{\partial x} & 0 \\ 0 & \frac{\partial N_1}{\partial y} & 0 & \frac{\partial N_2}{\partial y} & 0 & \frac{\partial N_3}{\partial y} \\ \frac{\partial N_1}{\partial y} & \frac{\partial N_1}{\partial x} & \frac{\partial N_2}{\partial y} & \frac{\partial N_2}{\partial x} & \frac{\partial N_3}{\partial y} & \frac{\partial N_3}{\partial x} \end{bmatrix} \{q\}_e = [B] \{q\}_e \\
 [B] &= \frac{1}{2A_e} \begin{bmatrix} b_1 & 0 & b_2 & 0 & b_3 & 0 \\ 0 & c_1 & 0 & c_2 & 0 & c_3 \\ c_1 & b_1 & c_2 & b_2 & c_3 & b_3 \end{bmatrix} \rightarrow \{\varepsilon\} = [B] \{q\}_e \text{ - strain is constant} \\
 & \{\sigma\} = [D] \{\varepsilon\} \text{ - stress is constant}
 \end{aligned}$$

CST – Constant Strain Triangle

Elastic strain energy in the CST element. Local stiffness matrix

elastic strain energy in a finite element:

$$U_e = \frac{1}{2} \int_{\Omega_e} [\varepsilon] \{\sigma\} d\Omega_e = \frac{1}{2} [\varepsilon] \{\sigma\} \int_{\Omega_e} d\Omega_e = \frac{1}{2} [q]_e [B]^T [D] [B] \{q\}_e A_e t_e =$$

$$= \frac{1}{2} [q]_e [k]_e \{q\}_e$$

$$[\varepsilon] = [q]_e [B]^T$$

1×3 1×6 6×3

$$\{\sigma\} = [D] \{\varepsilon\}$$

3×1 3×3 3×1

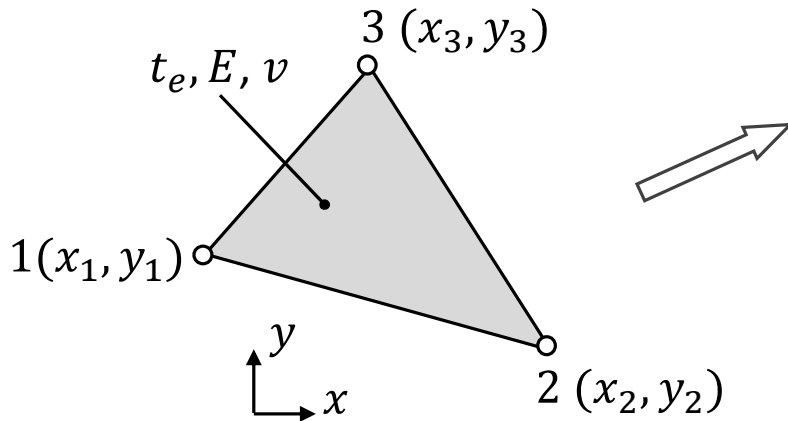
$$\{\varepsilon\} = [B] \{q\}_e$$

3×1 3×6 6×1

local stiffness matrix:

$$[k]_e = A_e t_e [B]^T [D] [B]$$

6×6 6×3 3×3 3×6



Potential energy of loading in the CST element

potential energy of loading in a finite element:

$$W_e = \int_{\Omega_e} [X] \{u\} d\Omega_e + \int_{\Gamma_{pe}} [p] \{u\} d\Gamma_{pe} =$$

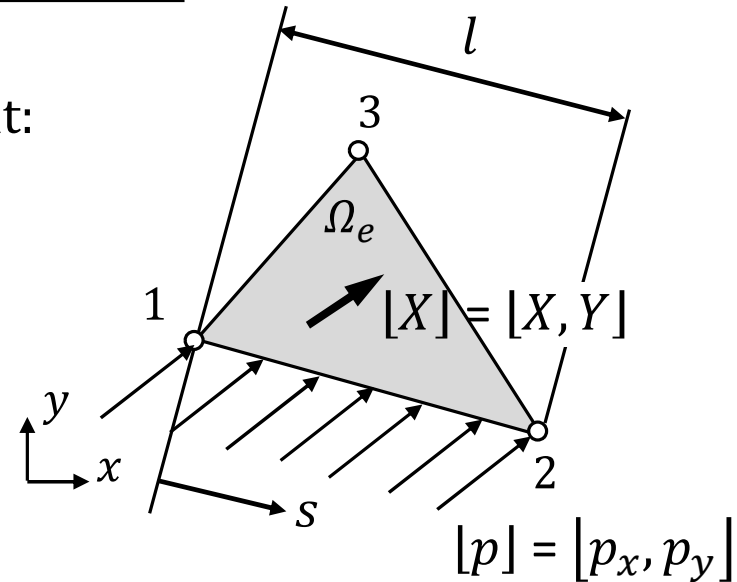
$$\{u\} = [N] \{q\}_e$$

$$= \int_{\Omega_e} [X][N] \{q\}_e d\Omega_e + \int_{\Gamma_{pe}} [p][N] \{q\}_e d\Gamma_{pe} =$$

$$= \left(\int_{\Omega_e} [X][N] d\Omega_e + \int_{\Gamma_{pe}} [p][N] d\Gamma_{pe} \right) \{q\}_e = ([F^X]_e + [F^p]_e) \{q\}_e = [F]_e \{q\}_e$$

$$[F^X]_e = t_e \int_{A_e} [X][N] dA_e \quad ;$$

$$[F^p]_e = t_e \int_0^l [p][N] ds$$



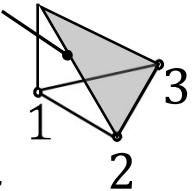
Components of equivalent load vector in the CST element

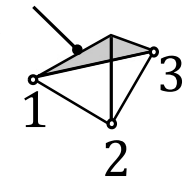
equivalent load vector due to mass forces:

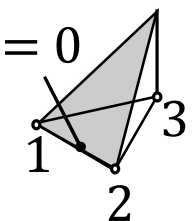
$$\begin{aligned}
 [F^X]_e &= t_e \int_{A_e} [X, Y] \begin{bmatrix} N_1 & 0 & N_2 & 0 & N_3 & 0 \\ 0 & N_1 & 0 & N_2 & 0 & N_3 \end{bmatrix} dA_e = \\
 &= t_e \int_{A_e} [XN_1, YN_1, XN_2, YN_2, XN_3, YN_3] dA_e = [F_{1e}^X, F_{2e}^X, F_{3e}^X, F_{4e}^X, F_{5e}^X, F_{6e}^X]
 \end{aligned}$$

equivalent load vector due to surface load:

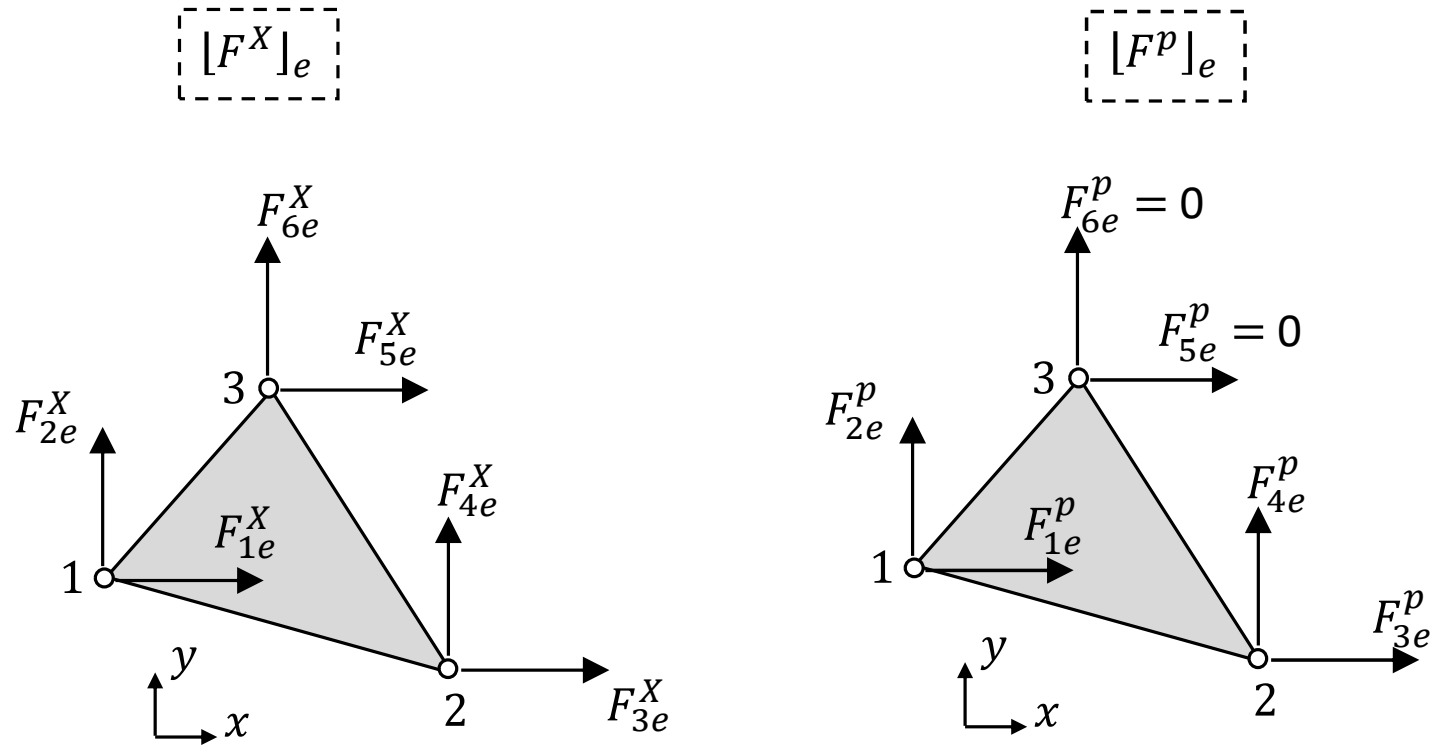
$$\begin{aligned}
 [F^p]_e &= t_e \int_0^l [p_x, p_y] \begin{bmatrix} N_1 & 0 & N_2 & 0 & N_3 & 0 \\ 0 & N_1 & 0 & N_2 & 0 & N_3 \end{bmatrix} ds = \\
 &= t_e \int_0^l [p_x, p_y] \begin{bmatrix} 1 - \frac{s}{l} & 0 & \frac{s}{l} & 0 & 0 & 0 \\ 0 & 1 - \frac{s}{l} & 0 & \frac{s}{l} & 0 & 0 \end{bmatrix} ds = \\
 &= t_e \int_0^l \left[p_x \left(1 - \frac{s}{l}\right), p_y \left(1 - \frac{s}{l}\right), p_x \frac{s}{l}, p_y \frac{s}{l}, 0, 0 \right] ds = \\
 &= [F_{1e}^p, F_{2e}^p, F_{3e}^p, F_{4e}^p, F_{5e}^p, F_{6e}^p]
 \end{aligned}$$

$$N_1(s)|_{1-2} = 1 - \frac{s}{l}$$


$$N_2(s)|_{1-2} = \frac{s}{l}$$


$$N_3(x, y)|_{1-2} = 0$$


Equivalent load vector in the CST element

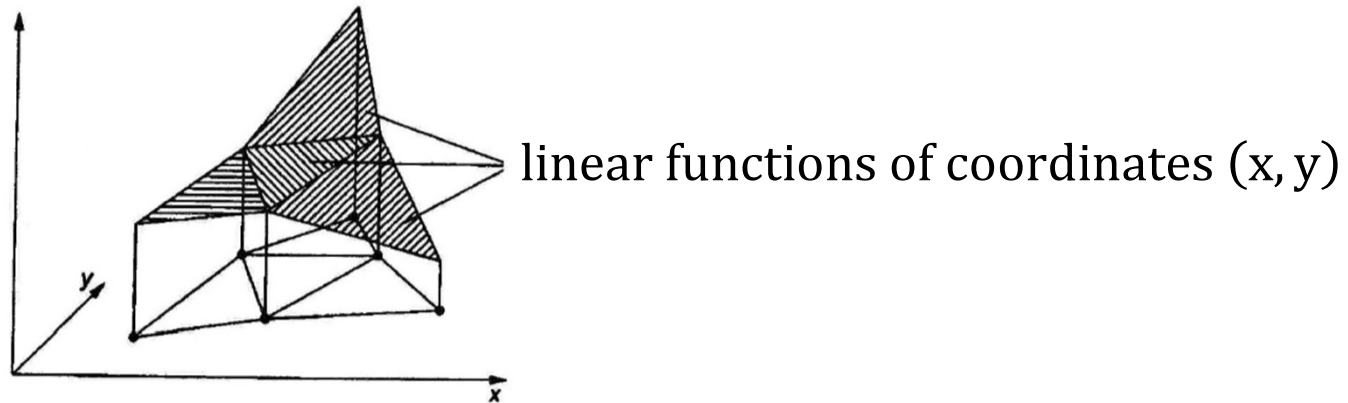


equivalent load vector:

$$[F]_e = [F_{1e}^X + F_{1e}^p, F_{2e}^X + F_{2e}^p, F_{3e}^X + F_{3e}^p, F_{4e}^X + F_{4e}^p, F_{5e}^X + F_{5e}^p, F_{6e}^X + F_{6e}^p]$$

Results in the CST element

DOF solution : $u(x, y), v(x, y)$



element solution: $\{\sigma\}, \{\varepsilon\}$
 $3 \times 1 \quad 3 \times 1$

